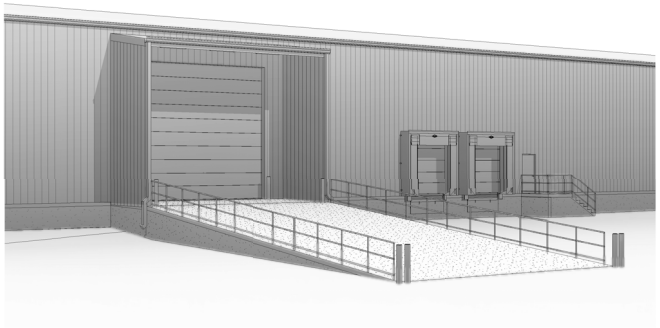
Cherokee Nation Businesses

MAIP Distribution Center Improvements

2777 OK-69A Pryor, OK 74361



Index of Drawings

Project Directory

Owner:
Cherokee Nation Businesses
777 West Cherokee Street
Catoosa, Oklahoma 74015
(918) 384-7735

Architect:
MGM Design Group
1820 S. Boulder Ave., Suite 401
Tulsa, Oklahoma 74119
(918) 269-6097

Structural Engineers:
Wallace Design Collective
123 N M.L.K. Blvd.
Tulsa, Oklahoma 74103
(918) 584-5858

Civil Engineers:
Route 66 Engineering
28 N Water St,
Sapulpa, OK 74066
(918) 584-5858

mgm design group

Cherokee Nation Businesses

MAIP Distribution Center Improvements

2777 OK-69A Pryor, OK 74361

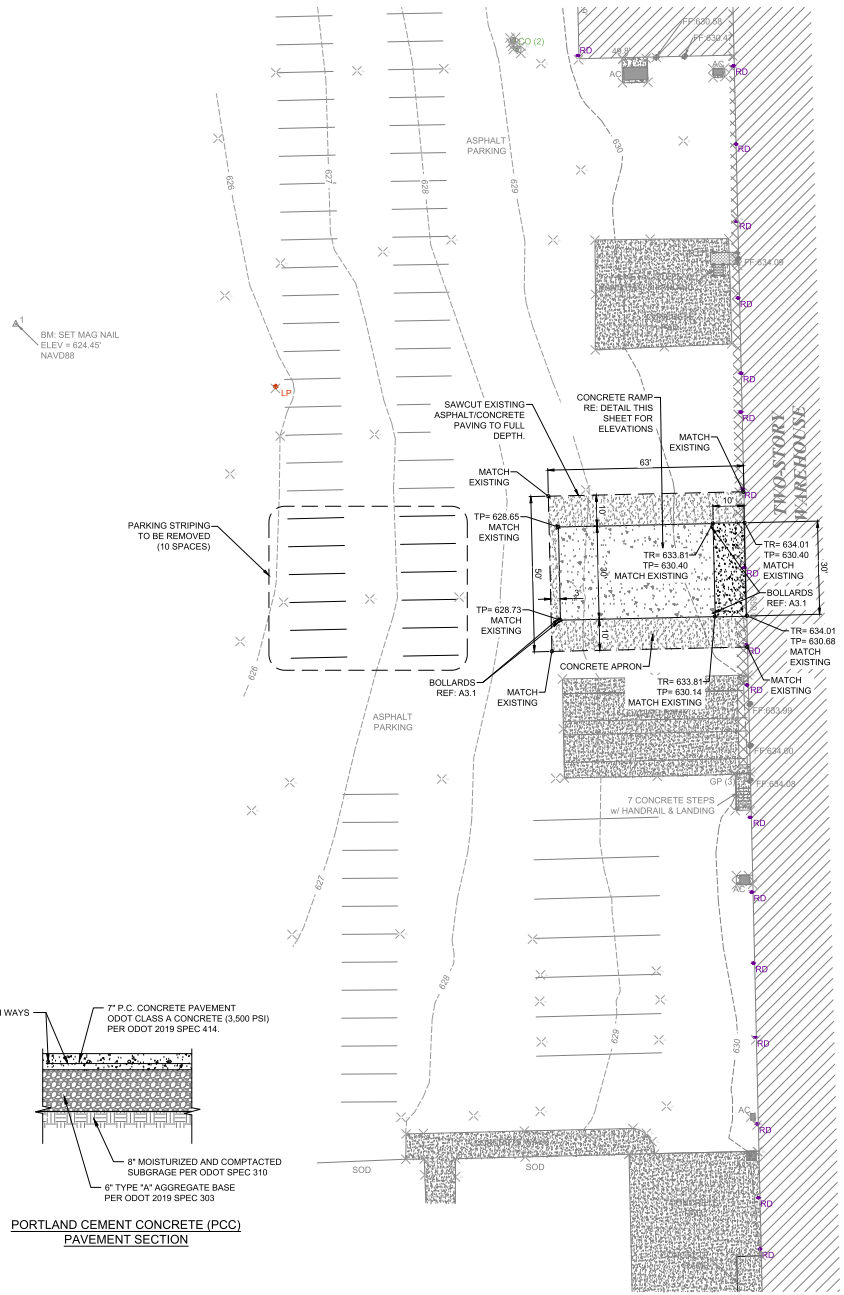
REVISIONS

STATE OF OKLAHOMA
F. LUBBOCK DAVIES III
2012
TULSA, OKLA
LICENSED ARCHITECT

DATE
12.04.23

SHEET
Cover Sheet

G1.0



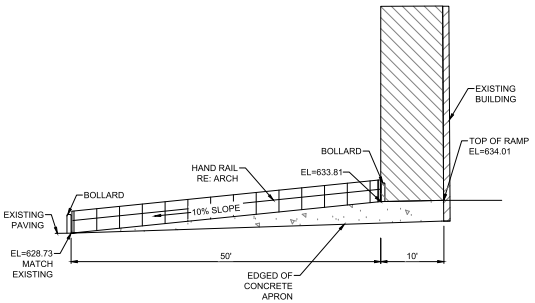
PORTLAND CEMENT CONCRETE (PCC) PAVEMENT SECTION

SITE PLAN NOTES

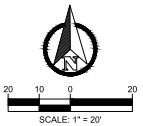
1. THE CONTRACTOR SHALL CONTACT "OKIE" AT 811 OF 800-522-6543, THREE (3) WORKING DAYS BEFORE BEGINNING AY WORK, SO EXISTING UNDERGROUND UTILITIES CAN BE LOCATED AND MARKED.
2. EQUIPMENT AND MATERIALS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
3. EARTHWORK FOR ALL BUILDING FOUNDATIONS AND SLABS SHALL BE IN ACCORDANCE WITH STRUCTURAL BUILDING PLANS AND SPECIFICATIONS AND THE GEOTECHNICAL REPORT FOR THIS PROJECT. REPORT PREPARED BY: AIMRIGHT TESTING & ENGINEERING PROJECT #9330821, DATED AUGUST 24, 2021
4. ALL CONSTRUCTION AND METHODS TO BE IN STRICT ACCORDANCE WITH CURRENT CITY OF PRYOR STANDARD SPECIFICATIONS AND DETAILS.
5. THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND WILL NOT BE LIMITED TO NORMAL WORKING HOURS. MAINTAIN ALL BARRICADES, WARNING SIGNS, FLASHING LIGHTS AND TRAFFIC CONTROL DEVICES DURING CONSTRUCTION. CONTRACTOR SHALL COMPLY WITH ALL O.S.H.A. REGULATIONS AND SAFETY REQUIREMENTS.
6. THIS SET OF CONSTRUCTION DOCUMENTS SHALL BE CONSIDERED AS A WHOLE IN THAT THE GENERAL CONTRACTOR AND ALL SUB-CONTRACTORS ARE RESPONSIBLE FOR INFORMATION PRESENTED ON ALL SHEETS OF THIS SET OF DRAWINGS.
7. CONTRACTOR IS TO BE RESPONSIBLE FOR OBTAINING ANY REQUIRED STATE OR LOCAL PERMITS. CONSTRUCTION MEANS AND METHODS SHALL COMPLY WITH ALL APPLICABLE GOVERNING CODES AND BE CONSTRUCTED TO SAME.
8. IF ANY EXISTING STRUCTURES TO REMAIN ARE DAMAGED DURING CONSTRUCTION, IT IS THE CONTRACTOR'S RESPONSIBILITY TO REPAIR AND/OR REPLACE THE EXISTING STRUCTURE, AS NECESSARY, TO RETURN IT TO THE EXISTING CONDITION OR BETTER. CONTRACTOR SHALL REPAIR AND RESTORE ANY AREAS DAMAGED DURING CONSTRUCTION AT HIS OWN EXPENSE.
9. THE CONTRACTOR SHALL PERFORM THE WORK ACCORDING TO ALL CITY, COUNTY, STATE AND FEDERAL SAFETY AND HEALTH REGULATIONS. IN PARTICULAR THE "TRENCHING" AND "OPEN EXCAVATION" OPERATIONS SHALL COMPLY WITH ALL CURRENT O.S.H.A. REGULATORY REQUIREMENTS.
10. ALL PAVEMENT MARKING OF STRIPES TO BE 4" WIDE, WHITE AND APPLIED IN TWO COATS, UNLESS OTHERWISE NOTED, RE: SPECIFICATIONS.

GRADING PLAN NOTES:

1. THE CONTRACTOR SHALL CONTACT "OKIE" AT 811 OR 800-522-6543, ONE CALL SERVICE, THREE (3) WORKING DAYS BEFORE BEGINNING ANY WORK, SO EXISTING UNDERGROUND UTILITIES MAY BE LOCATED AND MARKED.
2. EARTHWORK FOR ALL BUILDING FOUNDATIONS AND SLABS SHALL BE IN ACCORDANCE WITH STRUCTURAL BUILDING PLANS AND SPECIFICATIONS.
3. THE MAXIMUM CROSS SLOPE ON ANY SIDEWALK OR RAMP SHALL BE TWO PERCENT.
4. EQUIPMENT AND MATERIALS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
5. UNLESS OTHERWISE SHOWN, NEW PAVING SHALL BE CONSTRUCTED TO ALLOW FOR POSITIVE DRAINAGE TO CATCH BASIN, CURB, GUTTER, AND OTHER RUNOFF COLLECTION DEVICES. NEW PAVEMENT SLOPE SHALL BE MINIMUM 0.50% FOR CONCRETE AND 1.5% FOR ASPHALT UNLESS OTHERWISE INDICATED OR DIRECTED BY THE ARCHITECT/ENGINEER.
6. THE CONTRACTOR SHALL KEEP THE SITE CLEAN AT ALL TIMES AND CONTROL DUST RESULTING FROM THE EARTHWORK OPERATIONS. THE CONTRACTOR SHALL NOT TRACK MUD ON THE PUBLIC STREETS.
7. THE CONTRACTOR SHALL PROTECT EXISTING UTILITIES AND SHALL NOT DAMAGE OR DISTURB ANY SERVICE. THE CONTRACTOR SHALL REPAIR, AT CONTRACTOR'S OWN EXPENSE, ANY DAMAGED UTILITIES CAUSED BY CONSTRUCTION OPERATIONS.
8. IF FIELD GRADE ADJUSTMENTS ARE REQUIRED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER.
9. IF, AT ANY TIME THE CONTRACTOR FINDS ERROR AND/OR CONFLICTS IN THE CONTRACT DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER FOR CLARIFICATION PRIOR TO CONSTRUCTION.
10. THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CONTACT THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF EXISTING UTILITIES ON SITE OR IN RIGHT-OF-WAY. ALL UTILITIES MUST BE LOCATED PRIOR TO GRADING START.
11. SITE GRADING SHALL NOT PROCEED UNTIL APPROPRIATE EROSION AND SEDIMENT CONTROL MEASURES HAVE BEEN INSTALLED. THE CONTRACTOR SHALL ADHERE TO ALL TERMS AND CONDITIONS AND OUTLINED IN THE GENERAL NPDES PERMIT AND THE SWPPP FOR STORMWATER DISCHARGE ASSOCIATED WITH CONSTRUCTION ACTIVITIES.
12. THE CONTRACTOR SHALL ASSURE POSITIVE DRAINAGE AWAY FROM BUILDINGS FOR ALL NATURAL AND PAVED AREAS THROUGHOUT ALL PHASES OF CONSTRUCTION.



CONCRETE RAMP
SCALE: 1" = 10'



mgm design group

route 66 engineering
28 NORTH WATER STREET, SAPULPA, OK 74066

Cherokee Nation Businesses
MAIP Distribution Center Improvements
2777 OK-69A Pryor, OK 74361

REVISIONS

OKLAHOMA PROFESSIONAL ENGINEER BILLY COX 23063

BILLY COX, P.E.
ROUTE 66 ENGINEERING, LLC
CA #8853, DATE 06/30/2025

DATE
12.04.23

SHEET
SITE/GRADING PLAN

CS101

1 PARTIAL SITE PLAN
1" = 20'-0"
0 10' 20' 40'
NORTH

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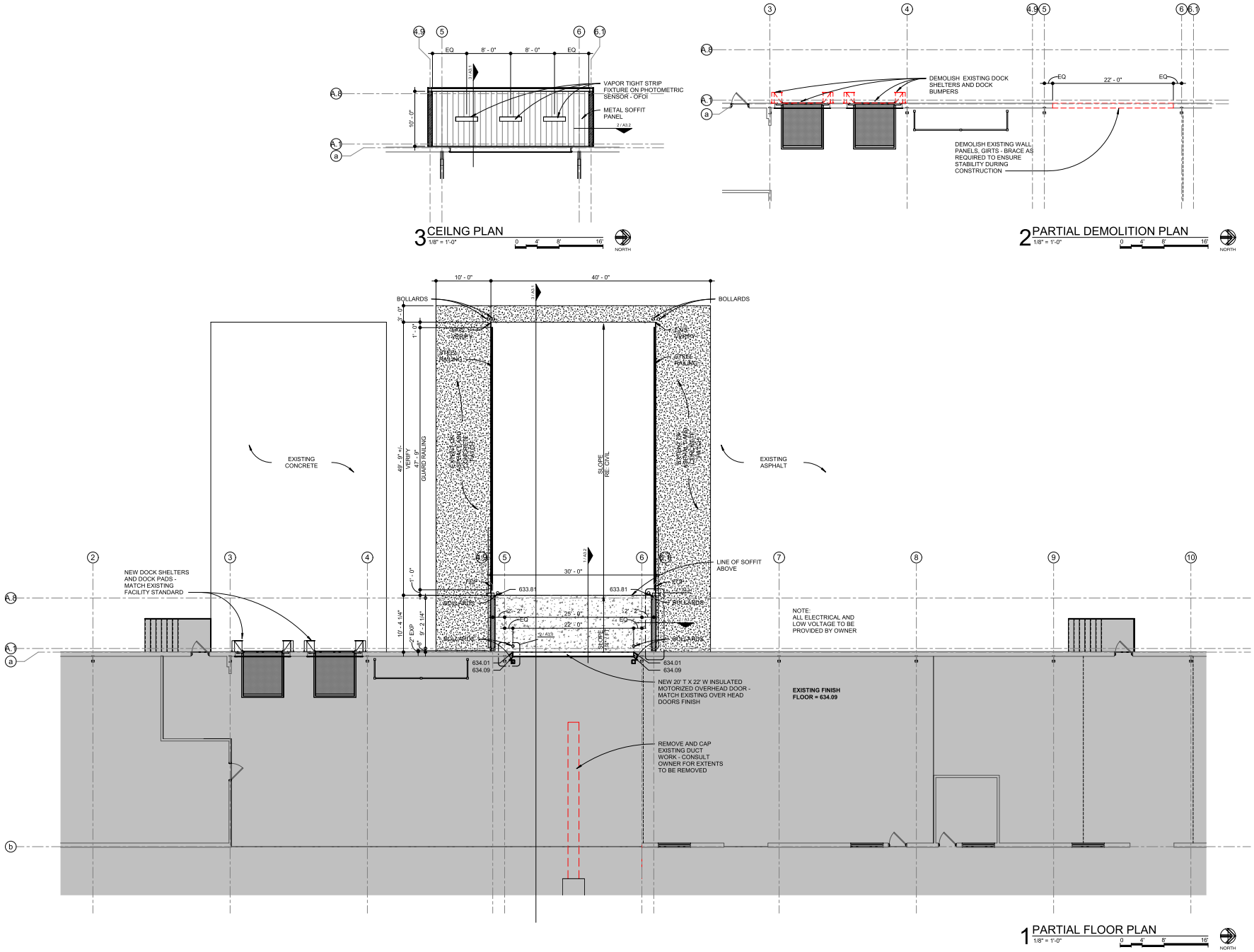
SHEET
Partial Architectural Site Plan

A1.0



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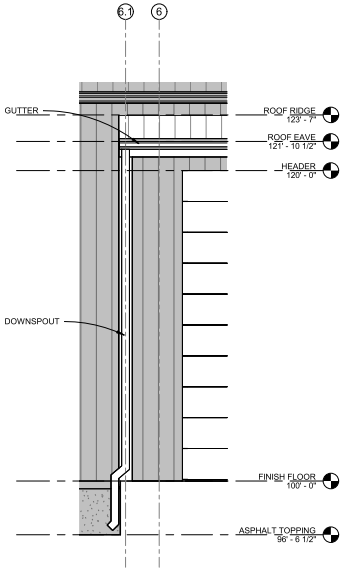
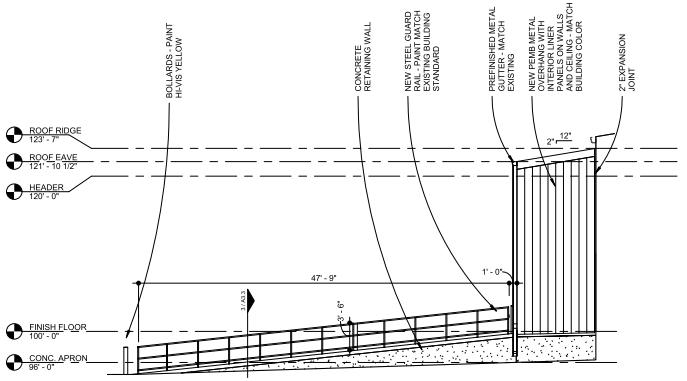
SHEET
Floor Plan

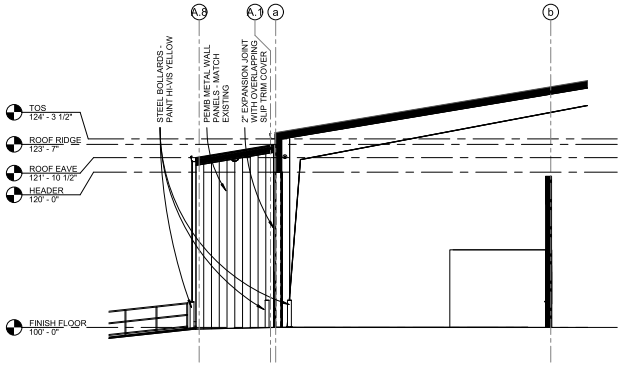
A2.2



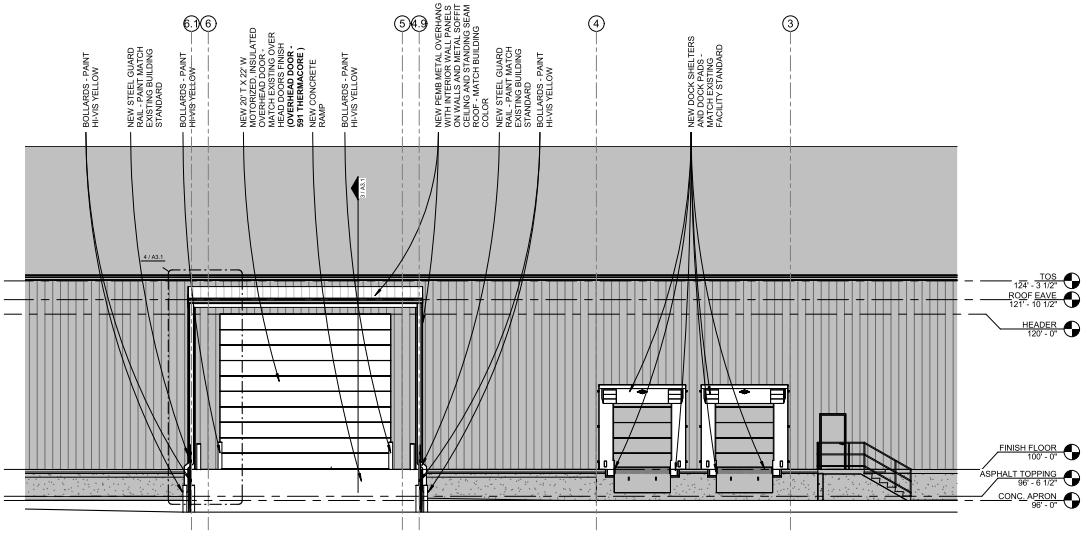
4 DOWSPOUT ELEVATION
1/4" = 1'-0"



2 PARTIAL SOUTH ELEVATION
1/8" = 1'-0"



BUILDING SECTION
3 LOOKING NORTH
1/8" = 1'-0"



1 PARTIAL WEST ELEVATION
1/8" = 1'-0"

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F. LURBOCK DAVIES III 2012 TULSA, OKLA
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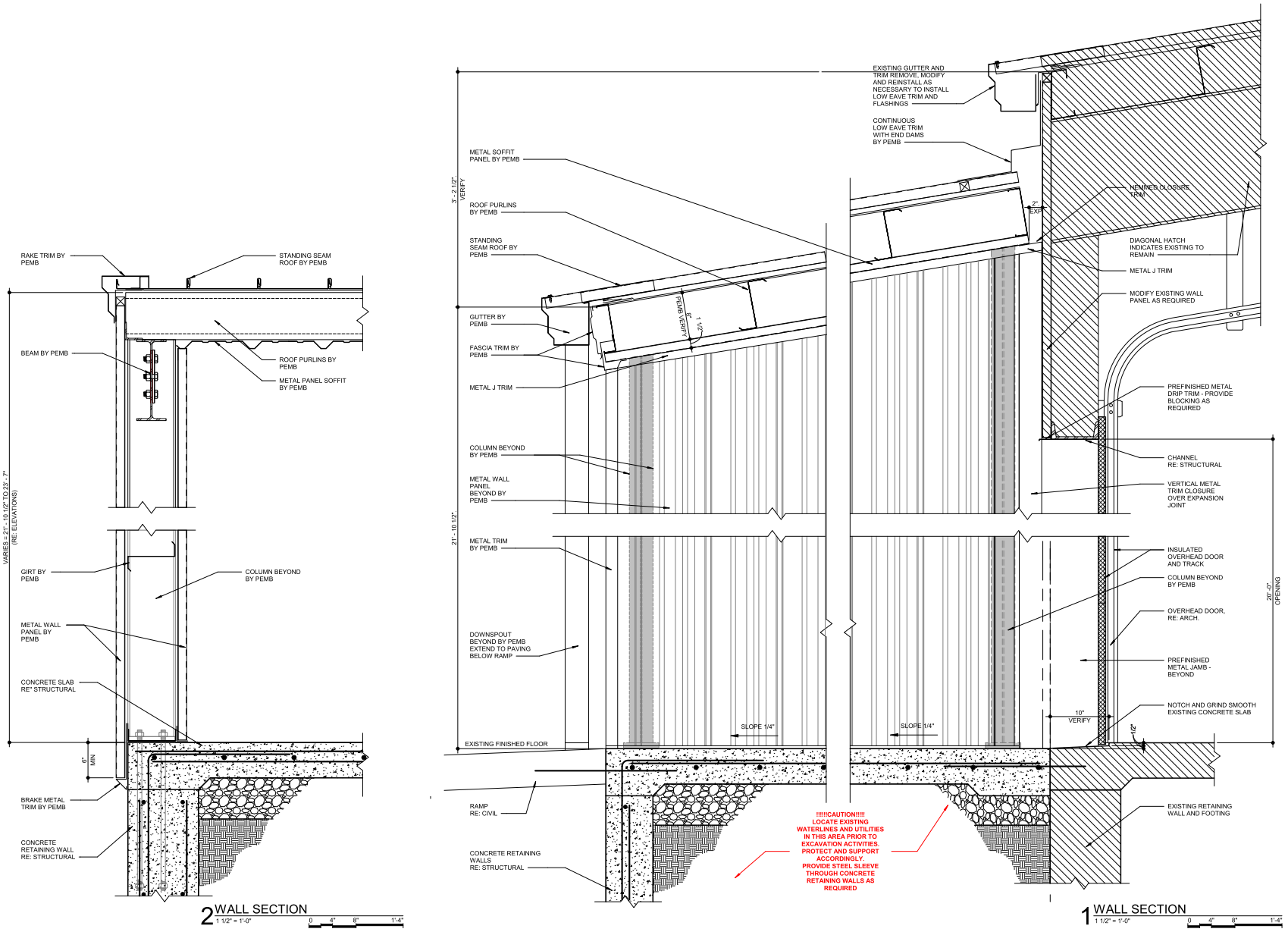
SHEET
Exterior Elevations

A3.1



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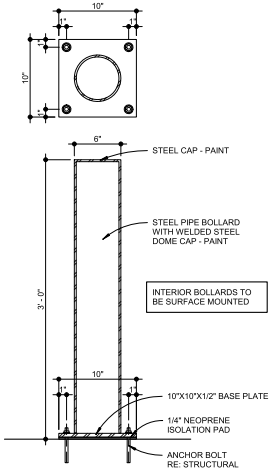
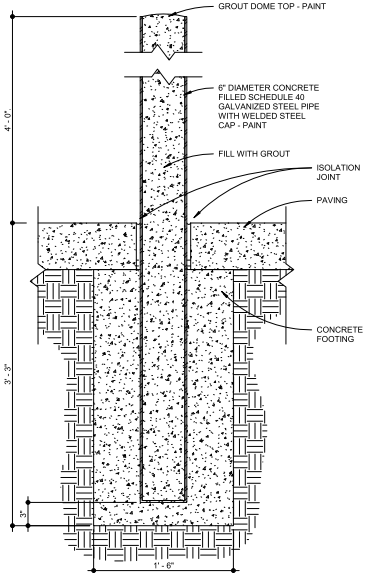
SHEET
Sections and Details

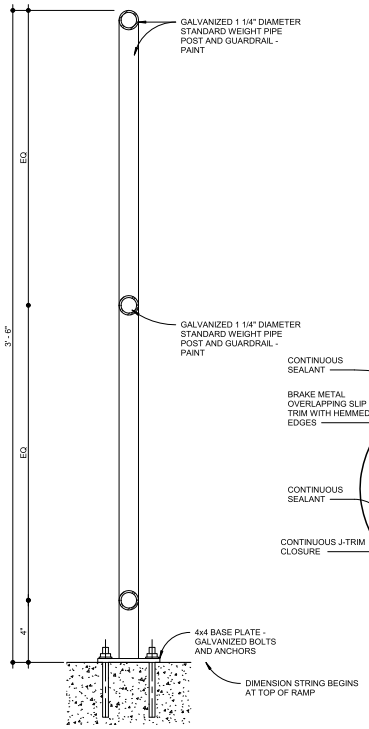
A3.2



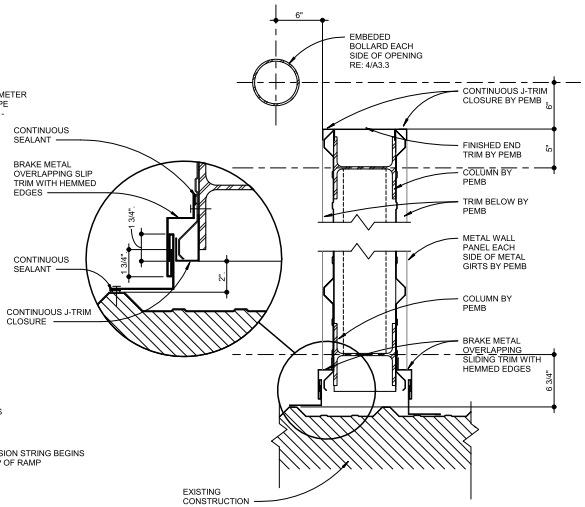
5 BOLLARD SECTION - SURFACE MOUNT (INTERIOR)
1 1/2" = 1'-0"



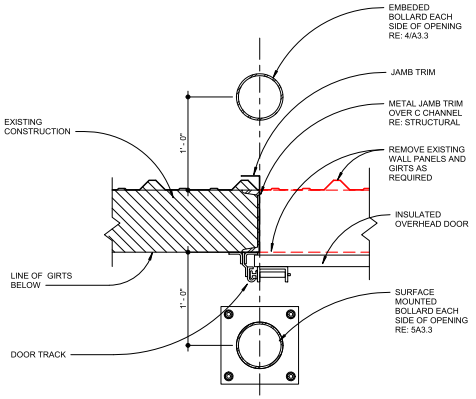
4 BOLLARD SECTION - EMBEDDED
1 1/2" = 1'-0"



3 RAILING SECTION
3" = 1'-0"



1 PLAN DETAIL
1 1/2" = 1'-0"



2 PLAN DETAIL
1 1/2" = 1'-0"

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SHEET
Details

A3.3

CODE AND DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
   - INTERNATIONAL BUILDING CODE, 2018 EDITION WITH OKLAHOMA STATE AMENDMENTS
2. STRUCTURE RISK CATEGORY
   - RISK CATEGORY II
3. GRAVITY LOADS

   3.1. DEAD LOAD — SELF WEIGHT OF DOOR AND FRAMING
4. WIND DESIGN DATA
   - BASIC DESIGN WIND SPEED — V = 108 MILES/HOUR
   - ALLOWABLE STRESS DESIGN WIND SPEED — V_{asd} = 84 MILES/HOUR
   - WIND EXPOSURE — EXPOSURE C
   - INTERNAL PRESSURE COEFFICIENT — GC_{pi} = +/- 0.18
   - SEE COMPONENTS AND CLADDING DESIGN WIND PRESSURE DIAGRAM
   - COMPONENTS AND CLADDING DESIGN WIND PRESSURES
     - WALLS
       - ZONE 4 — 13.1 PSF / -14.4 PSF — WHEN ZONE 5 DOES NOT APPLY.
       - ZONE 5 — 13.1 PSF / -15.7 PSF — 19'-6" FROM BUILDING CORNER IN EACH DIRECTION
     - COMPONENTS AND CLADDING WIND PRESSURES LISTED ABOVE ARE BASED UPON AN EFFECTIVE WIND AREA OF 10 SQUARE FEET. POSITIVE PRESSURES INDICATE WIND LOADING TOWARD THE SURFACE. NEGATIVE PRESSURES INDICATE WIND LOADING AWAY FROM THE SURFACE.
5. EARTHQUAKE DESIGN DATA
   - SEISMIC IMPORTANCE FACTOR — I_e = 1.00
   - MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS
     - 0.2-SECOND PERIOD — S_S = 0.127
     - 1.0-SECOND PERIOD — S_1 = 0.072
   - SITE CLASS — SITE CLASS D (ASSUMED DEFAULT)
   - DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS
     - 0.2-SECOND PERIOD — S_{DS} = 0.135
     - 1.0-SECOND PERIOD — S_{D1} = 0.115
   - SEISMIC DESIGN CATEGORY — SDC B
   - EXISTING BUILDING
     - BASED ON THE PROVISIONS OF CHAPTER 34 OF THE AMENDED INTERNATIONAL BUILDING CODE, STRUCTURAL ELEMENTS OF THE EXISTING STRUCTURE ARE NOT BEING ALTERED OR MODIFIED TO THE EXTENT REQUIRING THE EXISTING SEISMIC LATERAL FORCE RESISTING SYSTEM TO BE UPGRADED TO MEET THE PROVISIONS AND REQUIREMENTS OF THE CURRENT BUILDING CODE.
     - DEAD + LIVE LOAD — L/180 OR 2.0 INCHES

GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL, OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS, NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. THE CONTRACT DOCUMENTS INCLUDE BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR REFERENCE TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION, OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS UNLESS SPECIFICALLY STATED OTHERWISE.
4. THE CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI, OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. THE CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, MEMBER SIZES, AND SITE CONDITIONS BEFORE STARTING WORK. THE DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.
7. THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
8. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
9. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
10. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE CAUSED BY THE USE OF CONSTRUCTION EQUIPMENT ON THE STRUCTURE. ANY DAMAGE CAUSED BY CONSTRUCTION EQUIPMENT SHALL BE REPAIRED.
11. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR UNLESS PROVIDED FOR IN THE CONTRACT OR AS AGREED TO BY THE DESIGN TEAM AND THE CONTRACTOR.
12. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
13. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE LOCATIONS SPECIFICALLY INDICATED.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING MATERIAL STANDARDS, UNLESS NOTED OTHERWISE.
   - CHANNELS — ASTM A36
   - ANGLES — ASTM A36
   - PLATES, RODS, AND CONNECTING MATERIAL — ASTM A36
2. BOLTS AND ANCHORS:

   2.1. BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN THE SHEAR PLANE) WITH MINIMUM 1/2 INCH DIAMETER, ASTM F3125, GRADE A325 BOLTS.
3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS FOR REVIEW BY THE STRUCTURAL DESIGN PROFESSIONAL. REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS.

   4.1. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL DESIGN PROFESSIONAL.
5. ALL STRUCTURAL STEEL SHALL BE FABRICATOR'S STANDARD LEAD AND CHROMATE FREE -RUST INHIBITING PRIMER.

COLD-FORMED STEEL FRAMING

1. COLD-FORMED STEEL FRAMING AND CONNECTIONS ARE DESIGNED AND DETAILED TO CONFORM TO AISI 100 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
2. ALL STUDS, JOISTS, RACK, BRIDGING, END CLOSURES, AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF THE AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS UNLESS NOTED OTHERWISE.
3. PROVIDE COLD-FORMED STEEL FRAMING IN ACCORDANCE WITH THE FOLLOWING, UNLESS NOTED OTHERWISE
   - 54 MIL (16 GA) AND THICKER — ASTM A1003, GRADE 50, TYPE H (ST50H)
   - 43 MIL (18 GA) AND THINNER — ASTM A1003, GRADE 33, TYPE H (ST33H)
   - ACCESSORIES, TRACK, AND OTHER MEMBERS — ASTM A1003, GRADE 33, TYPE H (ST33H), MIN.
4. COLD-FORMED STEEL FRAMING MANUFACTURERS SHALL BE CERTIFIED BY THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) OR THE STEEL FRAMING INDUSTRY ASSOCIATION (SFIA).
5. UNLESS NOTED OTHERWISE, TRACKS SHALL BE 43 MILS (18 GA) MINIMUM THICKNESS FOR WALL STUDS 43 MIL (18 GA) OR THINNER. TRACKS SHALL MATCH WALL STUD THICKNESS FOR WALL STUDS 54 MIL (16 GA) AND THICKER.
6. CONNECTIONS SHALL CONSIST OF APPROVED FASTENERS AS SHOWN IN THE CONTRACT DOCUMENTS.
7. INSTALLATION OF BRIDGING MUST BE COMPLETE BEFORE ANY LOADS ARE APPLIED TO THE SYSTEM. TERMINATE BRIDGING AT JAMBS, CORNER STUDS, OR COLUMNS. BRACE STUDS AGAINST ROTATION.

EXISTING CONSTRUCTION CONDITIONS

1. WORK WITH EXISTING STRUCTURES REQUIRES THOROUGH COORDINATION OF THE CONTRACT DOCUMENTS WITH EXISTING CONDITIONS. THE CONTRACTOR MUST VERIFY ALL RELEVANT EXISTING CONDITIONS, DIMENSIONS, ELEVATIONS, DETAILS, ETC., BEFORE THE START OF WORK. THE CONTRACTOR MUST REPORT ANY DEVIATIONS FROM CONDITIONS OR DIMENSIONS SHOWN ON THE CONTRACT DOCUMENTS TO THE ARCHITECTURAL DESIGN PROFESSIONAL AND THE STRUCTURAL DESIGN PROFESSIONAL TO REVIEW THE DESIGN AND FOR POSSIBLE REVISION OF THE CONTRACT DOCUMENTS. BEGINNING FABRICATION MEANS ACCEPTANCE OF EXISTING CONDITIONS.
2. THE NATURE OF STRUCTURAL DEMOLITION OR STABILIZATION IS INHERENTLY UNCERTAIN. THE EXACT CONDITION AND CAPACITY OF EACH STRUCTURAL ELEMENT CANNOT BE VERIFIED BEFORE THE START OF WORK. IT IS IMPERATIVE TO REPORT ANY ELEMENT WITH QUESTIONABLE STRUCTURAL INTEGRITY TO THE ARCHITECTURAL DESIGN PROFESSIONAL AND THE STRUCTURAL DESIGN PROFESSIONAL FOR IMMEDIATE REVIEW.
3. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN AND ERECTION OF ALL SHORING, BRACING, AND PROTECTION MEASURES NECESSARY TO SAFEGUARD AND MAINTAIN THE EXISTING STRUCTURE DURING DEMOLITION AND CONSTRUCTION. THE CONTRACTOR SHALL SUBMIT A DETAILED PLAN FOR THE SHORING, BRACING, AND PROTECTION OF THE EXISTING CONSTRUCTION FOR REVIEW BY THE DESIGN PROFESSIONAL. THE REVIEW OF THE SUBMITTAL BY THE STRUCTURAL DESIGN PROFESSIONAL IS ONLY FOR GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE PLAN MUST INCLUDE THE PROPOSED CONSTRUCTION SEQUENCE. THE SHORING, BRACING, AND PROTECTION PLAN MUST BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT JURISDICTION.
4. DURING WELDING OR ANY OTHER CONSTRUCTION ACTIVITY THAT GENERATES SPARKS OR INTENSE HEAT, THE CONTRACTOR SHALL PROVIDE ADEQUATE FIRE PROTECTION TO THE EXISTING STRUCTURE AND CONTENTS.
5. NO REINFORCING SHALL BE CUT WITHOUT THE APPROVAL OF THE STRUCTURAL DESIGN PROFESSIONAL. ADDITIONAL REINFORCEMENT OF THE SLAB MAY BE REQUIRED FOR NEW PENETRATIONS. CLUSTERED PENETRATIONS MAY NEED TO BE SEPARATED OR REGROUPED DEPENDING ON THE CONFIGURATION OF THE SLAB REINFORCING.
6. PENETRATIONS ARE NOT PERMITTED IN PRIMARY STRUCTURAL MEMBERS (BEAMS AND COLUMNS) WITHOUT THE STRUCTURAL DESIGN PROFESSIONAL'S WRITTEN PERMISSION.

THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE TO THE EXISTING BUILDING.

POST-INSTALLED FASTENING AND ANCHORAGE IN CONCRETE AND MASONRY

1. PROVIDE POST-INSTALLED ANCHORS ONLY WHERE SPECIFIED IN THE CONSTRUCTION DOCUMENTS OR WHERE SPECIFICALLY APPROVED BY THE STRUCTURAL DESIGN PROFESSIONAL. SUBMIT PROPOSED POST-INSTALLED ANCHORING PRODUCTS BEFORE USE.
2. PROVIDE CARBON STEEL ANCHOR RODS FOR ADHESIVE ANCHORING SYSTEMS MADE OF MATERIAL CONFORMING TO ASTM A193, GRADE B7. PROVIDE STAINLESS STEEL ANCHOR RODS FOR ADHESIVE ANCHORING SYSTEMS MADE OF MATERIAL CONFORMING TO ASTM A193, GRADE B6.
3. PROVIDE POST-INSTALLED, ADHESIVE CONCRETE ANCHORS IN CRACKED AND UNCRACKED CONCRETE MEETING THE FOLLOWING CRITERIA:

   3.1. ADHESIVE ANCHOR SYSTEMS (ADHESIVES AND CONNECTING HARDWARE) SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ACI 355.4 AND/OR ICC_ES AC308 FOR USE IN CRACKED CONCRETE. ANCHOR SYSTEMS SHALL BEAR A VALID ICC_ES REPORT (OR EQUIVALENT).

   3.2. ADHESIVE ANCHOR SYSTEMS INSTALLED IN OVERHEAD OR UPWARDLY INCLINED ORIENTATIONS, AND ADHESIVE ANCHOR SYSTEMS RESISTING SUSTAINED TENSION LOADS SHALL BE INSTALLED BY INSTALLERS CERTIFIED IN ACCORDANCE WITH THE ACI/CSRI "ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM."

   3.3. THE INSTALLATION SHALL BE INSPECTED IN ACCORDANCE WITH THE SCHEDULE OF SPECIAL INSPECTIONS.

   3.4. THE MINIMUM EMBEDMENT LENGTH OF ANCHORS SHALL BE SIX TIMES THE ANCHOR DIAMETER UNLESS NOTED OTHERWISE.
4. PREPARE THE HOLE AND INSTALL THE ANCHORS IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. DO NOT CORE DRILL INSTALLATION HOLES WITHOUT THE APPROVAL OF THE STRUCTURAL DESIGN PROFESSIONAL.
5. POST-INSTALLED ANCHORS EXPOSED TO WEATHER SHALL BE GALVANIZED.
6. FIELD VERIFY THE LOCATION OF EXISTING REINFORCEMENT (INCLUDING POST-TENSIONING TENDONS WHERE APPLICABLE) IN EXISTING CONCRETE ELEMENTS PRIOR TO SUBMITTING ANY SHOP DRAWINGS SHOWING POST-INSTALLED ANCHORS. NOTIFY THE STRUCTURAL DESIGN PROFESSIONAL OF ANY CONFLICTS BETWEEN EXISTING REINFORCEMENT AND POST-INSTALLED ANCHORS.

   6.1. LOCATION OF EXISTING ELEMENTS MAY BE ESTABLISHED USING GROUND-PENETRATING RADAR (GPR), RADAR IMAGING, X-RAY SCANNING, OR ANY OTHER RELIABLE NON-DESTRUCTIVE METHOD.

ABBREVIATIONS

ARCH	ARCHITECT / ARCHITECTURAL
B.O.	BOTTOM OF
BRG	BEARING
CLR.	CLEAR
COL	COLUMN
CONC	CONCRETE
CONT.	CONTINUOUS
E.F.	EACH FACE
ELEV.	ELEVATION
EMBED	EMBEDMENT / EMBEDDED
ENG	ENGINEER / ENGINEERING
EOR	ENGINEER OF RECORD
EXIST	EXISTING
F.V.	FIELD VERIFY
FTG.	FOOTING
GC	GENERAL CONTRACTOR
HORIZ.	HORIZONTAL
HORIZ.	HORIZONTAL

ABBREVIATIONS

KSI	KIPS PER SQUARE INCH
LBS	POUNDS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
MAX	MAXIMUM
MIN	MINIMUM
O.C.	ON CENTER
OH	OPPOSITE HAND
PEMB	PRE-ENGINEERED METAL BUILDING
RE:	REFER TO
REINF	REINFORCING / REINFORCEMENT
SF	SQUARE FEET
SIM.	SIMILAR
T&B	TOP AND BOTTOM
T.O.	TOP OF
TYP.	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT.	VERTICAL

SPECIAL INSPECTION REQUIREMENTS (2018)

SPECIAL INSPECTIONS REQUIREMENTS (IBC 2018 CHAPTER 17)

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS PER SECTION 1704 OF THE IBC. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN THE PROJECT SPECIFICATIONS.
2. REPORT REQUIREMENTS SHALL CONFORM TO SECTIONS 1704.2.4 AND 1704.5 OF THE IBC. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.
3. THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR REGARDING INDIVIDUAL INSPECTION FOR ITEMS LISTED ON THE STATEMENT OF SPECIAL INSPECTIONS AND AS NOTED ON THE BUILDING DEPARTMENT APPROVED PLANS. ADEQUATE NOTICE AND ACCESS TO APPROVED PLANS SHALL BE PROVIDED SO THAT THE SPECIAL INSPECTOR HAS TIME TO BECOME FAMILIAR WITH THE PROJECT.
4. FABRICATORS OF STRUCTURAL LOAD-BEARING OR LATERAL LOAD RESISTING MEMBERS OR ASSEMBLIES SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1704.2.5 OF THE IBC.
5. SPECIAL INSPECTION REPORTS AND A FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.

IBC 2018 REQUIRED SPECIAL INSPECTIONS

		FREQUENCY OF INSPECTION	
		CONTINUOUS	PERIODIC
STEEL CONSTRUCTION - STRUCTURAL STEEL (IBC SECTION 1705.2.1)			
1.	SPECIAL INSPECTION AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360-16.		

AISC 360-16, CHAPTER N SPECIAL INSPECTION REQUIREMENTS

		FREQUENCY OF INSPECTION	
		PERFORM	OBSERVE
N5.6 - INSPECTION OF HIGH-STRENGTH BOLTS			
AISC 360-16, TABLE N5.6-1 - INSPECTION TASKS PRIOR TO BOLTING			
1.	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	X	---
2.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	---	X
3.	CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH) IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE	---	X
4.	CORRECT BOLTING PROCEDURES SELECTED FOR JOINT DETAIL	---	X
5.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	---	X
6.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	X	X
7.	PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	---	X
AISC 360-16, TABLE N5.6-2 - INSPECTIONS DURING BOLTING			
1.	FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED	---	X
2.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	---	X
3.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	---	X
4.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	---	X
N5.7 - OTHER INSPECTION TASKS			
1.	INSPECTION OF GALVANIZED STEEL STRUCTURAL MAIN MEMBERS EXPOSED CUT SURFACES OF GALVANIZED MAIN MEMBERS AND EXPOSED CORNERS OF HSS SHALL BE VISUALLY INSPECTED FOR CRACKS SUBSEQUENT TO GALVANIZING.	X	---
N5.8 - OTHER INSPECTION TASKS			
1.	INSPECT THE STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS.	X	---
2.	INSPECT THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED AND DOCUMENTED PRIOR TO PLACEMENT OF CONCRETE	X	---
	** PERFORM - PERFORM THESE TASKS FOR EACH CONNECTION.		
	** OBSERVE - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.		

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SHEET: GENERAL NOTES AND SPECIAL INSPECTIONS

S0.1

CAST-IN-PLACE CONCRETE

1. MINIMUM COMPRESSIVE STRENGTH (F'C) AT THE END OF 28 DAYS SHALL BE AS FOLLOWS:

   A. FOUNDATION WALLS AND PEDESTALS — 4500 PSI, AIR ENTRAINED

   B. EXTERIOR SLABS — 4500 PSI, AIR ENTRAINED

   C. FOOTINGS — 3000 PSI, NON-AIR ENTRAINED

2. CONCRETE SHALL HAVE A MAXIMUM W/CM RATIO OF 0.45 AND 0.58, FOR 4000 PSI AND 3000 PSI CONCRETE, RESPECTIVELY.

3. NEITHER MATERIALS NOR ADMIXTURES SHALL CONTAIN ANY CALCIUM CHLORIDE.

4. SLUMP OF CONCRETE SHALL NOT EXCEED 4" UNLESS A HIGH RANGE WATER-REDUCING ADMIXTURE IS USED. THE SLUMP OF CONCRETE PRIOR TO ADDITION OF A HIGH RANGE WATER-REDUCING ADMIXTURE SHALL NOT EXCEED 4". THE SLUMP OF CONCRETE CONTAINING A HIGH RANGE WATER REDUCING ADMIXTURE SHALL NOT EXCEED 8".

5. CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE BETWEEN 4 AND 6 PERCENT.

6. COARSE AGGREGATE SIZE SHALL BE NO. 57 OR LARGER.

7. THE MINIMUM PORTLAND CEMENT CONTENT (ASTM C150) OF ALL CONCRETE SHALL CONFORM TO THE FOLLOWING TABLE.NORMALWEIGHT STRUCTURAL CONCRETE:

   SPECIFIED COMPRESSIVE STRENGTH — MINIMUM CEMENT CONTENT (POUNDS PER CUBIC YARD)

(PSI)	NON-AIR ENTRAINED CONCRETE	AIR ENTRAINED CONCRETE
• 4500	564	611
• 3000	470	---

8. FLY ASH MAY BE USED IN CEMENT MIXTURES WITH A MAXIMUM FLY ASH CONTENT EQUAL TO 20 PERCENT OF CEMENT CONTENT BY WEIGHT.

9. CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR REVIEW BY THE PROJECT MANAGER WELL IN ADVANCE OF CONCRETE PLACEMENT. CONCRETE MIX DESIGN SHALL INCLUDE ALL STRENGTH DATA NECESSARY TO SHOW COMPLIANCE WITH THE PROJECT SPECIFICATIONS FOR EITHER THE TRIAL BATCH OR FIELD EXPERIENCE METHOD, IN ACCORDANCE WITH ACI 301 (LAST EDITION).

10. IMMEDIATELY AFTER PLACEMENT, PROTECT CONCRETE FROM PREMATURE DRYING, EXCESSIVELY HOT OR COLD TEMPERATURES AND MECHANICAL DAMAGE. CURE IN ACCORDANCE WITH ACI 308.

11. ALL EMBEDDED ITEMS IN CONCRETE SHALL BE INSPECTED.

12. REFERENCES:

    ACI 211.1, RECOMMENDED PRACTICE FOR SELECTING PROPORTIONS FOR NORMAL AND HEAVYWEIGHT CONCRETE

    ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS

    ACI 302, GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION

    ACI 304, RECOMMENDED PRACTICE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE

    ACI 305, HOT WEATHER CONCRETING

    ACI 306, COLD WEATHER CONCRETING

    ACI 308, PRACTICE FOR CURING CONCRETE

    ACI 309, CONSOLIDATION OF CONCRETE

    ACI 318, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE SP15, FIELD REFERENCE MANUAL, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS WITH SELECTED ACI AND ASTM REFERENCES

13. 13. REINFORCING STEEL SHALL MEET THE FOLLOWING:

    - DEFORMED BARS — ASTM A615, GRADE 60
    - WELDABLE DEFORMED BARS — ASTM A706, GRADE 60
    - WELDED WIRE FABRIC — ASTM A185
    - STEEL FIBERS — ASTM A820

14. WHERE DOWELS ARE INDICATED BUT NOT SIZED, PROVIDE DOWELS THAT MATCH SIZE AND LOCATION OF MAIN REINFORCING STEEL AND LAP SPLICE WITH THE MAIN REINFORCING STEEL. REINFORCING BARS SHALL BE SPLICED AS NOTED IN THE REINFORCING LAP SCHEDULE.

15. REFER TO ACI 318 LATEST EDITION FOR CONCRETE COVER, ACI 315 LATEST EDITION FOR DETAILING PRACTICES AND FABRICATION, AND ACI 301 LATEST EDITION FOR STANDARD PRACTICE FOR MIXING AND PLACING CONCRETE.

16. "C.J." INDICATES SAW CUT CONTRACTION JOINT, "CONST. JT." INDICATES DOWELED CONSTRUCTION JOINT IN SLAB-ON-GRADE. REFERENCE SPECIFICATIONS FOR ACCEPTED SAW CUT METHODS. SLAB POURS SHALL BE SEPARATED BY A DOWELED CONSTRUCTION JOINT. CONTRACTION/CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN ON PLANS OR AS DIRECTED BY THE STRUCTURAL ENGINEER.

17. PROVIDE CORNER BARS THAT MATCH CONTINUOUS REINFORCEMENT SIZE AND QUANTITY AT INTERSECTIONS AND CORNERS OF WALLS AND FOUNDATIONS.

18. PROVIDE #3 Z-BAR SPACERS AT 24 INCHES ON CENTER EACH WAY FOR CONCRETE WALLS HAVING REINFORCING STEEL IN BOTH FACES.

19. ANCHORS INSTALLED IN HARDENED CONCRETE SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. ANCHORS MUST BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS AND APPLICABLE ESR REPORT. USE HILTI HIT RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM (OR HILTI HIT HY 150 MAX-SD ADHESIVE ANCHORING SYSTEM OR HILTI KWIK BOLT TZ EXPANSION ANCHOR). REFERENCE DETAILS FOR ANCHOR SIZE AND EMBEDMENT. SUBSTITUTIONS TO THE SPECIFIED ANCHORS MUST HAVE AN APPLICABLE ESR REPORT AND BE APPROVED BY ENGINEER OF RECORD

METAL BUILDING SYSTEM

1. THE METAL BUILDING SYSTEM ELEMENTS SHALL BE DESIGNED BY THE MANUFACTURER AND SHALL COMPLY WITH THE REQUIREMENTS OF LOCAL BUILDING CODES AS LISTED IN "DESIGN PARAMETERS" AND THE METAL BUILDING MANUFACTURERS' ASSOCIATION DESIGN MANUAL. IN ADDITION, THE METAL BUILDING ELEMENTS SHALL BE DESIGNED FOR A MINIMUM COLLATERAL LOAD OF 5.0 PSF (7.5 PSF WHERE CEILINGS ARE PRESENT) IN ADDITION TO THE WEIGHT OF ALL ROOF TOP MECHANICAL UNITS AND EQUIPMENT, AND ELECTRICAL CABLE TRAY LINES. REFER ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR WEIGHT AND LOCATION OF ALL MECHANICAL EQUIPMENT AND CABLE TRAY LINES SUPPORTED BY THE ROOF STRUCTURE.

2. THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR PROVIDING THE MATERIAL TYPE, DIAMETER, AND LOCATION OF ANCHOR RODS FOR THE METAL BUILDING COLUMNS.

3. THE METAL BUILDING COLUMNS SHALL BEAR AS INDICATED IN THE CONTRACT DOCUMENTS.

4. LIMIT LATERAL DEFLECTIONS OF FRAMES TO THE BUILDING EAVE HEIGHT DIVIDED BY 60.

5. LIMIT LATERAL DEFLECTIONS OF GIRTS TO THE SPAN DIVIDED BY 120.

6. LIMIT VERTICAL DEFLECTION OF PRIMARY ROOF FRAMING MEMBERS TO THE SPAN DIVIDED BY 180 (240 IF SUPPORTING CEILINGS) FOR LIVE LOAD, AND TO THE SPAN DIVIDED BY 120 (L/180 IF SUPPORTING CEILINGS) FOR TOTAL LOAD. AT SUPPLEMENTAL SUPPORT FRAMING, THE STRICTER OF THE PREVIOUSLY LISTED DEFLECTIONS AND THE MANUFACTURER'S DEFLECTION REQUIREMENTS SHALL CONTROL THE DESIGN.

7. LIMIT VERTICAL DEFLECTION OF PURLINS TO THE SPAN DIVIDED BY 150 FOR LIVE LOAD (240 IF SUPPORTING CEILINGS) AND TO THE SPAN DIVIDED BY 120 FOR TOTAL LOAD.

8. SECONDARY FRAMING SHALL ACCOMMODATE DEFLECTION OF PRIMARY FRAMING AND CONSTRUCTION TOLERANCES AND MAINTAIN CLEARANCES AT OPENINGS.

9. METAL BUILDING MANUFACTURER SHALL SUBMIT STRUCTURAL DRAWINGS AND CALCULATIONS PREPARED, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE MANUFACTURER SHALL PROVIDE UNFACTORED FOUNDATION REACTIONS FOR EACH LOAD TYPE (DEAD LOAD, LIVE LOAD, WIND LOAD, ETC.).

10. FOUNDATIONS PROVIDING SUPPORT TO THE METAL BUILDING FRAMES OF THE BUILDING HAVE BEEN DESIGNED FOR PINNED TYPE CONNECTIONS ONLY. DO NOT FIX THE BASE OF THE COLUMNS.

11. THE METAL BUILDING MANUFACTURER SHALL BE RESPONSIBLE FOR ALL FRAMING ABOVE SLAB. THIS INCLUDES, BUT IS NOT LIMITED TO, WIND GIRTS AND COLUMNS, EXTERIOR JAMBS AND LINTELS, SPECIALTY EQUIPMENT AND MECHANICAL/ELECTRICAL EQUIPMENT SUPPORT. ALL SUPPLEMENTAL FRAMING SHALL MEET OR EXCEED THE LOAD AND DEFLECTION REQUIREMENTS OF THE EQUIPMENT OR COMPONENT MANUFACTURER.

12. THE METAL BUILDING MANUFACTURER SHALL BE RESPONSIBLE FOR COORDINATING METAL BUILDING ELEMENTS WITH THE CONTRACT DOCUMENTS.

13. NO OVERSTRESS OF METAL BUILDING MEMBERS IS ALLOWED.

14. FOUNDATION DESIGN PRELIMINARY. CONSTRUCTION OF FOUNDATIONS SHALL NOT BEGIN UNTIL WALLACE DESIGN COLLECTIVE HAS REVIEWED AND APPROVED THE METAL BUILDING DESIGN AND COLUMN REACTIONS.

FOUNDATIONS

1. FOOTING DESIGNS ARE BASED ON AN ASSUMED STABLE, NON-EXPANSIVE SOIL WITH AN ALLOWING BEARING PRESSURE OF [1,500] PSF. THE CONTRACTOR SHALL HIRE A REGISTERED GEOTECHNICAL ENGINEER LICENSED IN THE STATE THE PROJECT IS LOCATED TO DETERMINE WHETHER OR NOT THE SOIL MEETS THE MINIMUM CRITERIA.

2. THE SOILS SUPPORTING THE FOUNDATION AND SLAB SHALL BE PREPARED AND COMPACTED IN ACCORDANCE WITH THE RECOMMENDATIONS FROM THE GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER SHALL VERIFY CONFORMANCE OF EXCAVATION, SCARIFYING, PROOF-ROLLING, FILL CLASSIFICATION, MAXIMUM PARTICLE SIZE, LIQUID LIMIT, PLASTICITY INDEX AND PLACEMENT PROCEDURES.

3. THE BEARING MATERIALS SHALL BE FREE OF ORGANIC, EXPANSIVE, OR CORROSIVE MATERIAL AND SHALL SUPPORT THE FOUNDATION IN ACCORDANCE WITH THE FOLLOWING CRITERIA:

   A. MAXIMUM DIFFERENTIAL SETTLEMENT SHALL NOT EXCEED [1/2"] OVER A DISTANCE OF [50] FEET

   B. MAXIMUM TOTAL MOVEMENT DUE TO EITHER SETTLEMENT OR HEAVE SHALL NOT EXCEED [1"]

4. IF THE CRITERIA CANNOT BE MET, THE ENGINEER OF RECORD SHALL BE NOTIFIED SO THAT THE FOUNDATION MAY BE REDESIGNED ACCORDINGLY.

5. EXTERIOR FOOTINGS SHALL BEAR AT OR BELOW MINIMUM BEARING DEPTH. MINIMUM BEARING DEPTH IS 2 FEET BELOW THE LOWEST ADJACENT GRADE.

6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING EXACT QUANTITIES OF CUT AND FILL FOR ESTIMATING AND CONSTRUCTION.

7. FOUNDATION WALLS SHALL HAVE ADEQUATE TEMPORARY BRACING INSTALLED BY THE CONTRACTOR BEFORE BACKFILL IS PLACED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL WALL IS PERMANENTLY BRACED.

8. AVOID DAMAGE TO UNDERGROUND UTILITIES INCLUDING, BUT NOT LIMITED TO, WATER MAINS, SANITARY SEWERS AND BURIED CABLES WHICH MIGHT EXTEND ACROSS OR ADJOIN SITE.

DEFERRED STRUCTURAL SUBMITTALS (IBC 2018 SECTION 107.3.4.1)

1. THE FOLLOWING STRUCTURAL COMPONENTS SHALL BE DESIGNED AND SUBMITTED BY OTHERS FOR APPROVAL IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

   A. METAL BUILDING STRUCTURE.

2. DOCUMENTS FOR DEFERRED STRUCTURAL SUBMITTAL ITEMS SHALL BE DESIGNED, SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER-OF-RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL AS REQUESTED WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED FOR DESIGN LOADS AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN CRITERIA OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

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EXP DATE: 06/30/25

REGISTERED PROFESSIONAL ENGINEER — MONCHER LABBANE — 18532 — OKLAHOMA — 12.4.23

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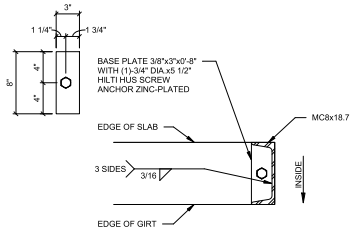
REVISIONS

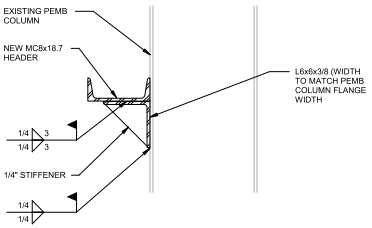
DATE
12.04.23

SHEET
GENERAL NOTES

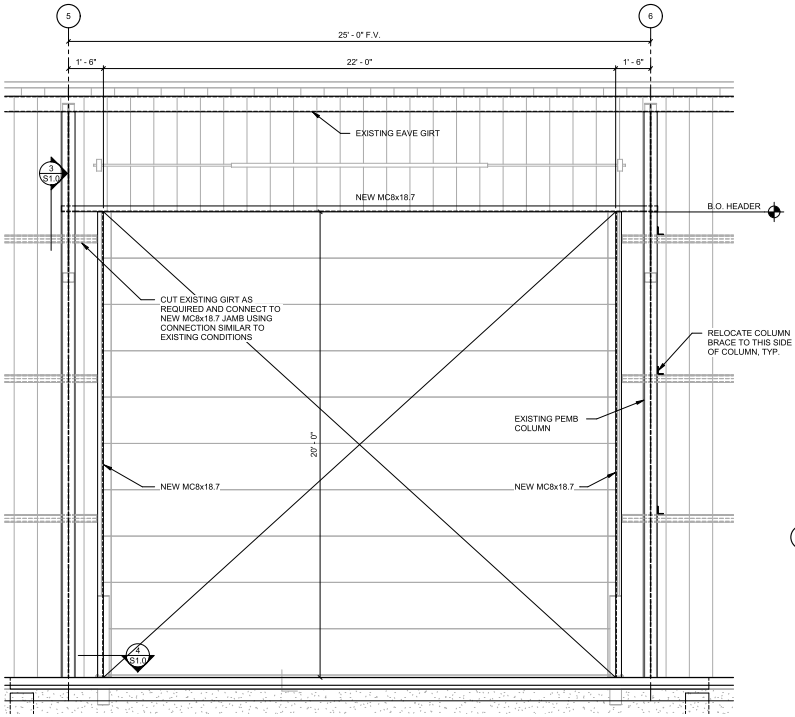
S0.2



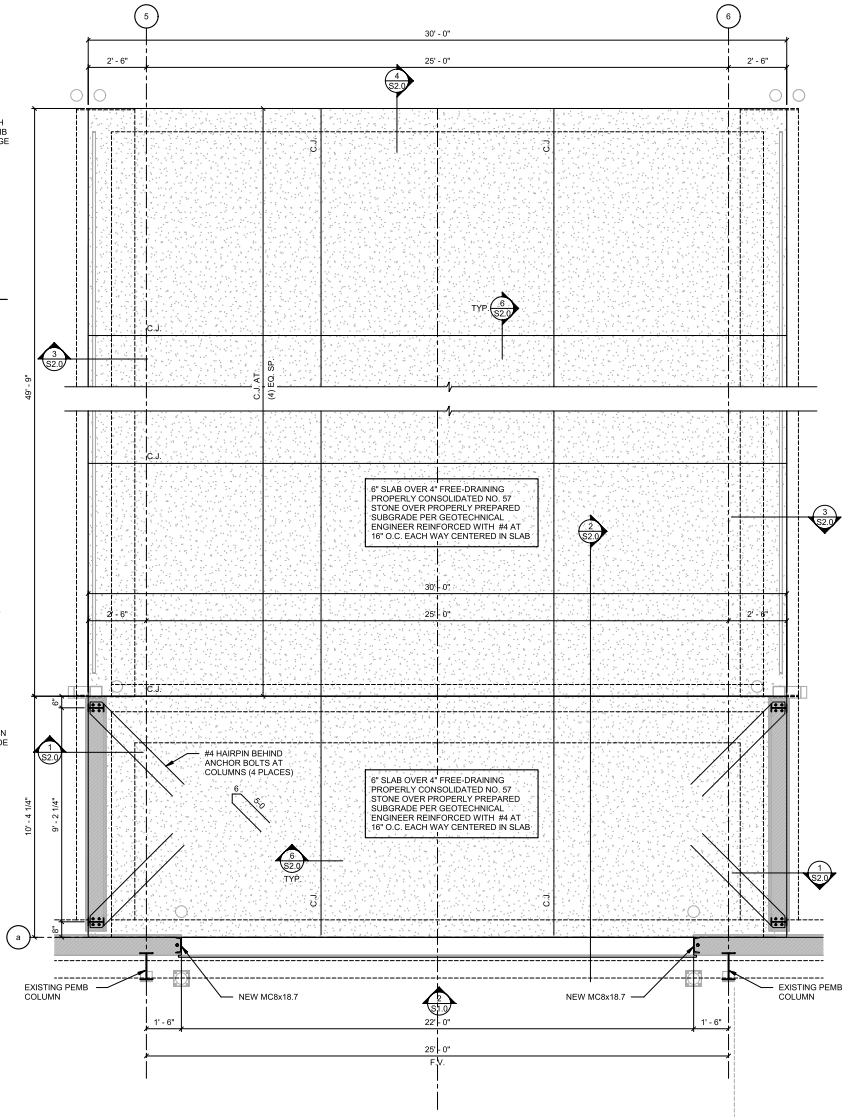
4 JAMB CONNECTION DETAIL AT BASE
1 1/2" = 1'-0"



3 HEADER CONNECTION DETAIL
1 1/2" = 1'-0"



2 INTERIOR ELEVATION AT NEW DOOR OPENING
3/8" = 1'-0"



1 FOUNDATION PLAN
3/8" = 1'-0"

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SHEET
PLAN AND DETAILS

S1.0

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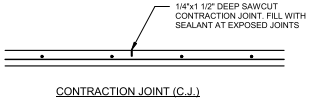
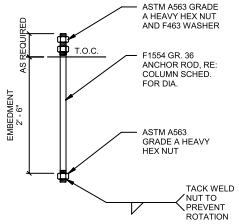
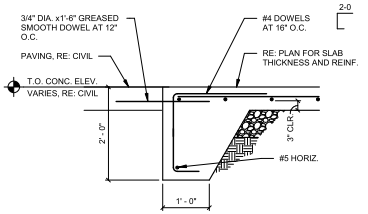


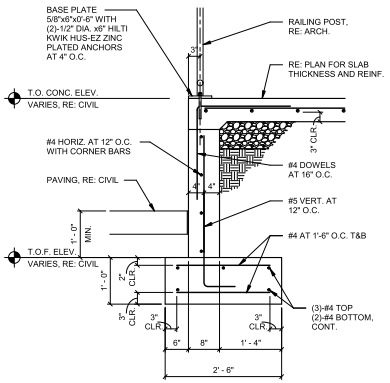
6 CONTRACTION JOINT DETAIL
3/4" = 1'-0"



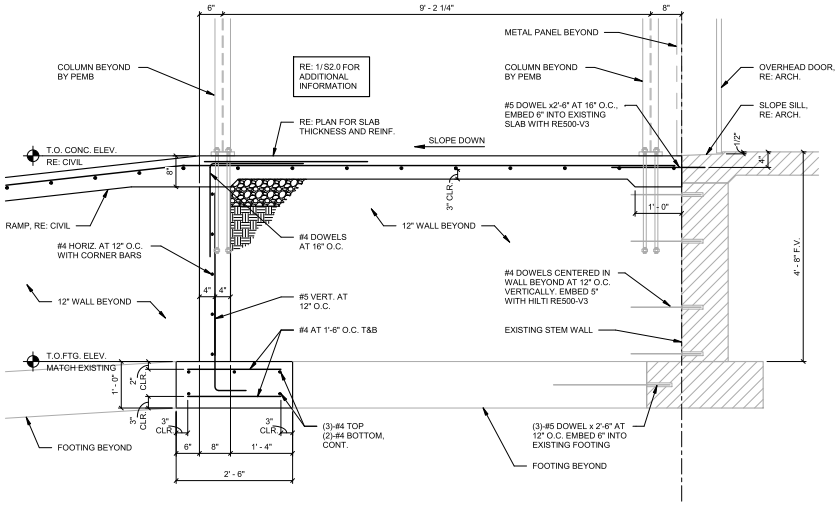
5 ANCHOR BOLT DETAIL
3/4" = 1'-0"



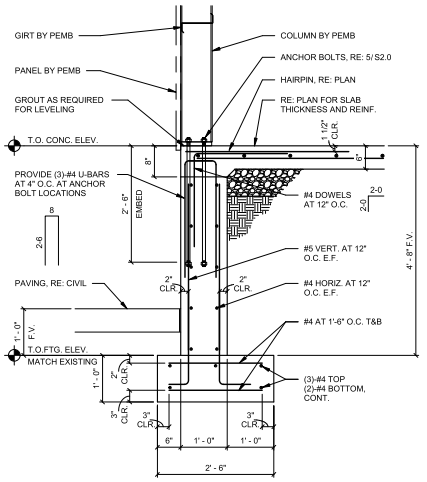
4 FOUNDATION SECTION
3/4" = 1'-0"



3 FOUNDATION SECTION
3/4" = 1'-0"



2 FOUNDATION SECTION
3/4" = 1'-0"



1 FOUNDATION SECTION
3/4" = 1'-0"

DATE 12.04.23
SHEET DETAILS

S2.0